

## SIMPLIFY CHARTS FOR THE DESIGN OF VERTICAL FORMWORK SYSTEMS

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*This paper presents simplify charts for the design of traditional wall formwork systems. The first step is to determine the pressure due to fresh concrete using the cement type , concrete weight , rate of placing and the temperature .The second step is to determine the safe distance between studs using plywood thicknesses 19 or 22 or25 mm .The third step is to determine the distance between walls using studs of 50\*100 mm (2\*4 in) or 50\*125 mm (2\*5 in) or 50\*150 mm (2\*6 in) lumber .Finally the safe distance between ties is determined using wale section of double 50\*100mm (2\*4 in) or 50\*125 mm (2\*5 in) lumber.*

**KEYWORDS:** wall formwork, construction.

### NOTATIONS

$A$	area of section (mm <sup>2</sup> )*	$F_v$	allowable unit stress in horizontal shear (kPa)
$b$	width of member (mm)	$f_c$	actual unit stress in compression parallel to grain (kPa)
$d$	depth of member (mm)	$F_{cI}$	actual unit stress in compression perpendicular to grain (kPa)
$E$	modulus of elasticity (kPa)	$f_t$	actual unit stress in tension (kPa)
$EI$	plywood stiffness capacity (kPa mm <sup>4</sup> /m)	$I$	moment of inertia (mm. <sup>4</sup> )*
$F_b$	allowable unit stress in bending (kPa)	$l$	length of span, center to center of supports (mm)
$F_{bKS}$	plywood section capacity in bending (Nmm/m)	$P$	applied force (compression or tension) (lb)
$F_c$	allowable unit stress in compression parallel to grain (kPa)	$S$	section modulus (mm. <sup>3</sup> )*
$F_{cI}$	allowable unit stress in compression perpendicular to grain (kPa)	$w$	uniform load per foot of span (kPa/m)
		$\Delta$	deflection (mm)

\* For a rectangular member:  $A = bd$ ,  $S = bd^2/6$ ,  $I = bd^3/12$ .

### INTRODUCTION

An inherent property of concrete is that it can be transformed into any shape. The wet mixture is placed in forms constructed of wood, metal or other suitable material to harden or set. The form must be assembled with quality workmanship, holding to close

dimensional tolerances. Formwork should be strong enough to support the concrete's weight and rigid enough to maintain its position and appearance. In addition, formwork must be tight enough to prevent the water seepage and designed to permit ready removal. Because the formwork for a concrete structure constitutes a considerable item in the cost of the completed structure, particular care should be exercised in its design. It is desirable to maintain a repetition of identical units so that the forms may be removed and reused at other locations with a minimum amount of labor.

Formwork holds concrete until it sets, produces the desired shapes, and develops a desired surface finish. Forms also protect concrete, aid in curing, and support any reinforcing bars or conduit embedded within the concrete. Because formwork can represent up to one-third of a concrete structure's total cost, this phase of a project is very important. The nature of the structure, availability of equipment and form materials, anticipated reuse of the forms, and familiarity with construction methods all influence the formwork design. To design forms, the strength of the forming materials and the loads they support you must know. The concrete's final shape, dimensions, and surface finish must be considered.

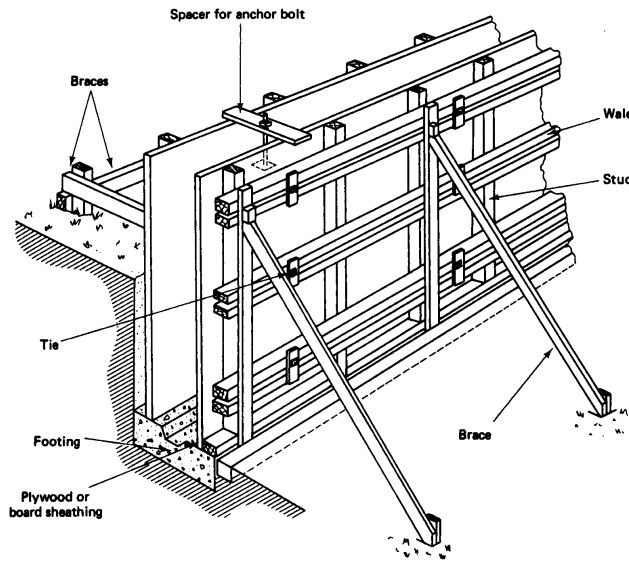


Fig (1) Traditional wall formwork system

## TRADITIONAL DESIGN CALCULATIONS FOR WALL FORMWORK SYSTEMS

### STEP 1: Determination of pressure due to fresh concrete

The first step is to determine the pressure due to fresh concrete using the concrete unit ( $\gamma$ ) weight, cement type, rate of placing (R) and the temperature (T) .using the formula shown in Table (1):

Table (1) Lateral pressure due to fresh concrete on wall formwork

<b>R &lt; 2.1 m/hr</b>	<b>2.1 ≤ R ≤ 4.6 m/hr</b>	<b>R &gt; 4.6 m/hr</b>
(1) $P = C_w C_c ( 7.2 + \frac{785 R}{T + 18} ) \dots\dots\dots(1)$	(1) $P = C_w C_c ( 7.2 + \frac{1154}{T + 18} + \frac{244 R}{T + 18} ) \dots\dots\dots (2)$	$P = \gamma h$
(2) $P_{\max} = \gamma h$	(2) $P_{\max} = \gamma h$	
(3) $P_{\min} = 28.7 C_w$ (kPa)	(3) $P_{\min} = 28.7 C_w$ (kPa)	

Where :

P = pressure due to fresh concrete (kPa)

R = rate of placing (m/hr)

T = temperature (°C)

 $C_w$  = concrete unit weight coefficient as shown in Table (2) $C_c$  = concrete chemistry coefficient as shown in Table (3)

h = wall height (m)

 $\gamma$  = concrete unit weight ( $\text{kg/m}^3$ )Table (2) Concrete unit weight coefficient ( $C_w$ )

$\gamma$	$C_w$
Under $2243 \text{ kg/m}^3$	$0.5(1 + \frac{\gamma}{2323})$ but at least 0.80
2243 to $2403 \text{ kg/m}^3$	1.0
Over $2403 \text{ kg/m}^3$	$(\frac{\gamma}{2323})$

Where :

 $\gamma$  = concrete unit weight  $\text{kg/m}^3$ Table (3) Concrete chemistry coefficient ( $C_c$ )

<b>Cement Type or Blend</b>	<b><math>C_c</math></b>
Type I, II, or III without retarders	1.0
Type I, II, or III with a retarders	1.2
Other blends containing less than 70% slag or 40% fly ash without retarders	1.2
Other blends containing less than 70% slag or 40% fly ash with a retarder	1.4
Other blends containing more than 70% slag or 40% fly ash	1.4

### Step 2 Determination of safe distance between studs:

The safe distance between studs is determined using plywood sheathing of thickness 19 or 22 or 25 mm. The allowable stresses and section properties of such plywood is determined from the manufacture's catalog. Then the safe distance is determined to satisfy both bending (Equation1) and deflection (Equation2) for three or more spans as follows (See Appendix I for notations :

The safe distance due to bending is:

$$l = 3.16 \left( \frac{F_b K S}{w} \right)^{1/2} \quad \dots\dots(1)$$

The safe distance due to deflection (l/360) is:

$$l = \frac{73.8}{1000} \left( \frac{EI}{w} \right)^{1/3} \quad \dots\dots(2)$$

### Step 3 Determination of safe distance between walls:

The safe distance between walls is determined using the section properties of 50\*100 mm (2\*4 in) ,or 50\*125 mm ( 2\*5 in) or 50\*150mm ( 2\*6 in) cross sections . The cross sectional area, section modulus, and moment of inertia are determined for the proposed section then the adjusted allowable stress for lumber being used is determined. The safe distance between walls is determined to satisfy the bending (Equation 3) and deflection (Equation 4) for case for three or more spans as follows (See Appendix I for notations):

The safe distance due to bending is:

$$l = \frac{40.7}{1000} d \left( \frac{F_b b}{w} \right)^{1/2} \quad \dots\dots(3)$$

The safe distance due to deflection (l/360) is:

$$l = \frac{73.8}{1000} \left( \frac{EI}{w} \right)^{1/3} \quad \dots\dots(4)$$

### Step 4 Determination of safe distance between ties:

The safe distance between ties is determined using the section properties of double walls of 50 \*100 mm (2\*4 in) o 50\*125 mm (2\*5 in) cross sections . The cross sectional area , section modulus , and moment of inertia is determined for the proposed section then the adjusted allowable stress for lumber being used is determined . The safe distance between ties is determined to satisfy the bending from Equation (3) and deflection from Equation (4) for case for three or more spans.

## HOW TO USE THE SIMPLIFIED CHARTS TO DESIGN TRADITIONAL WALL FORMWORK SYSTEMS

In order to demonstrate the design using simplified charts, a case example was performed using the following data:

Wall height 3.0 m, rate of placing 1.5 m/hr, temperature is 27°C, cement type I without retarders, concrete unit weight is 2403 kg/m<sup>3</sup>, plywood sheathing is 25 mm thickness

Class (I), studs will be 50\*100 mm and walls will double 50\*100 mm. The use of these simplified charts is as follows:

- (1) Referring to Fig. (2) to determine the pressure due to fresh concrete if rate of placing less than 2.1 m/hr , Fig.(3) if rate of placing greater or equal 2.1 but less or equal 4.6 m/hr and if rate of placing greater than 4.6 m/hr the pressure will be  $\gamma h$  . For rate of placing equal 1.5 m/hr entering at the bottom of chart (Fig (2) point A) to the line representing the expected concrete temperature, the pressure due to fresh concrete is in the vertical axis (point B) .
- (2) Referring to Fig (4) find the safe distance between studs is determine by drawing a vertical line from point C across to the curve for the desired plywood thickness 25 mm (point D) (N.B. point D is the minimum distance resulting from bending and deflection).
- (3) Referring to Figs (5,6,7) find distance between walls .Use Fig (5) if the studs will be 50\*100 mm (2\*4 in) , Fig(6) if the studs will be 50\*125 mm (2\*5 in) , and Fig(7) if the stud will be 50\*150 mm (2\*6 in.) .In our case the studs will be 50\*100 mm so, Fig (5) will be used. From point E draw a vertical line (distance between studs ) to intersect the desired pressure (point F).(N.B. the distance in this curve is the minimum distance resulting from bending and deflection point G)
- (4) Referring to Figs (7,8) find distance between metal ties. Use Fig (7) if the walls will be double 50\*100 mm (2\*4 in) , Fig(8) if the walls will be double 50\*125 mm (2\*5 in) . In our case the walls will be double 50\*100 mm so Fig (7) will be used. From point H draw a vertical line (distance between walls) to intersect the desired pressure (point I).(N.B. the distance in this curve is the minimum distance resulting from bending and deflection point (J).

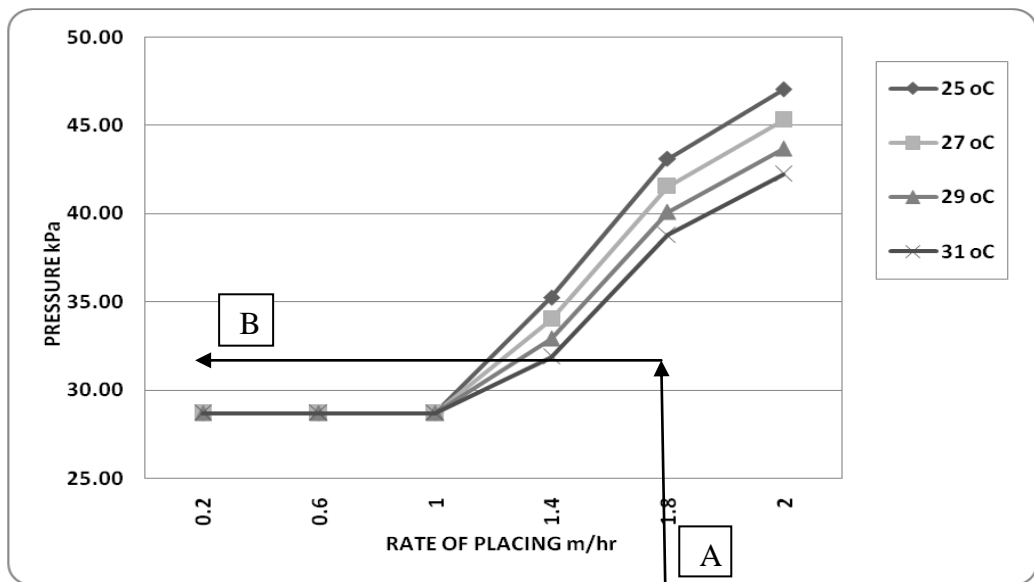


Fig. (2) Determination of pressure due to fresh concrete for  $R < 2.1$  m/hr

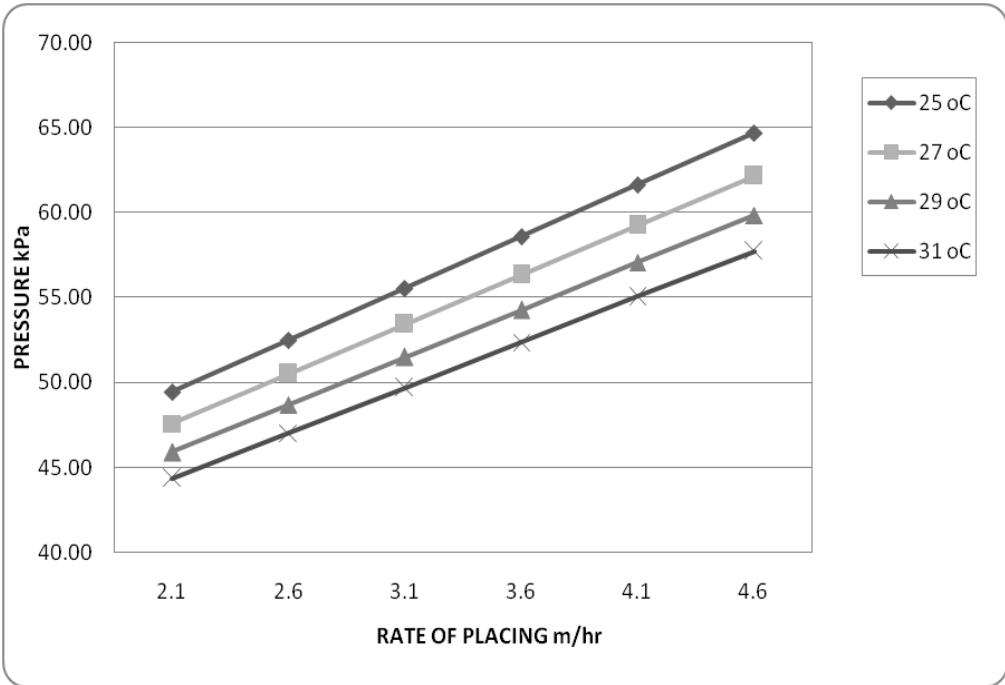


Fig. (3) Determination of pressure due to fresh concrete for  $2.1 \leq R \leq 4.6$  m/hr

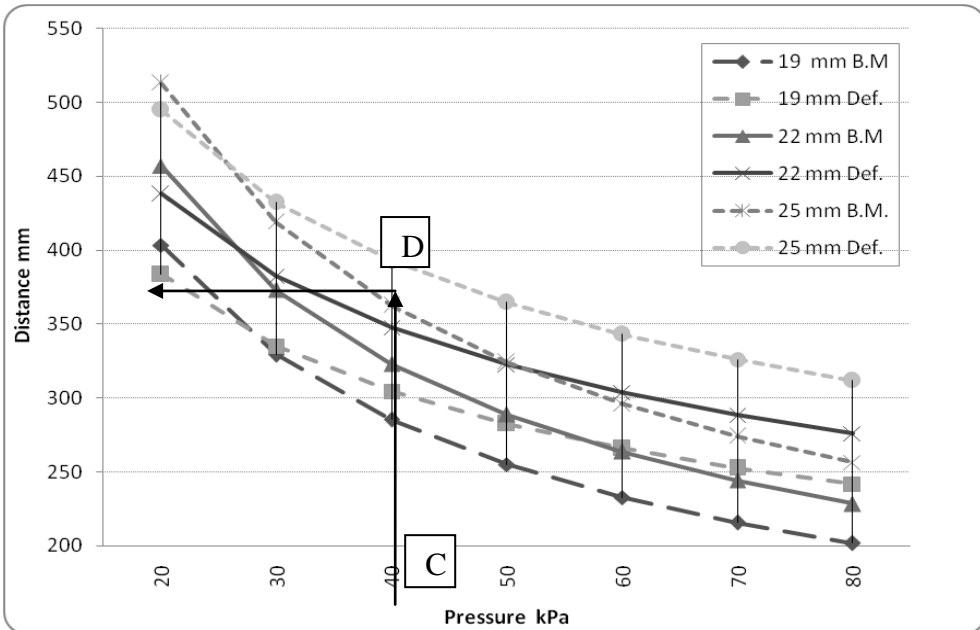


Fig. (4) Determination of safe distance between studs

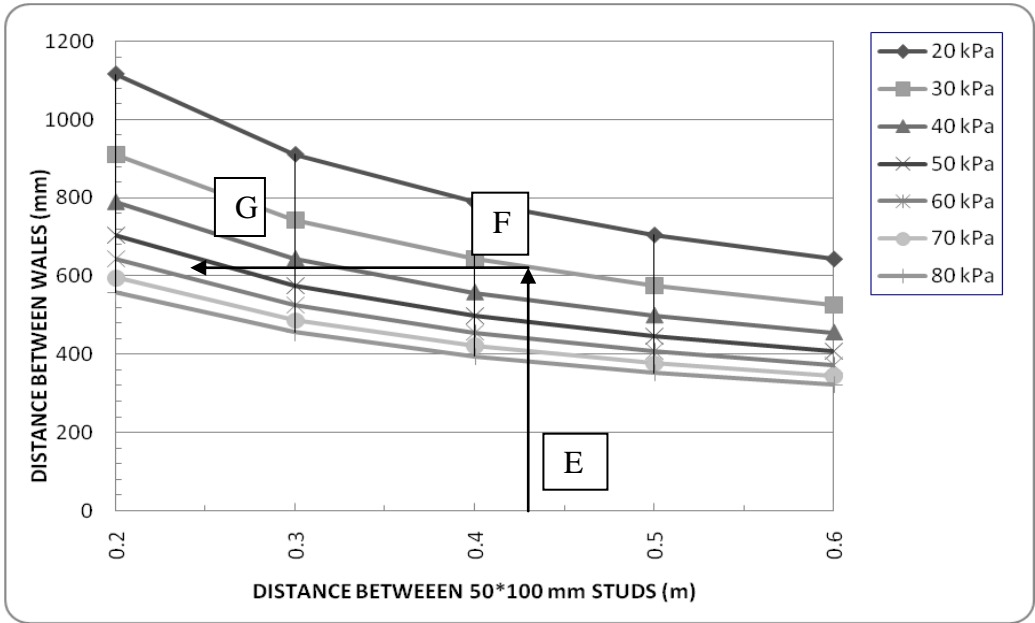


Fig. (5) Determination of safe distance between walls (50\*100 studs)

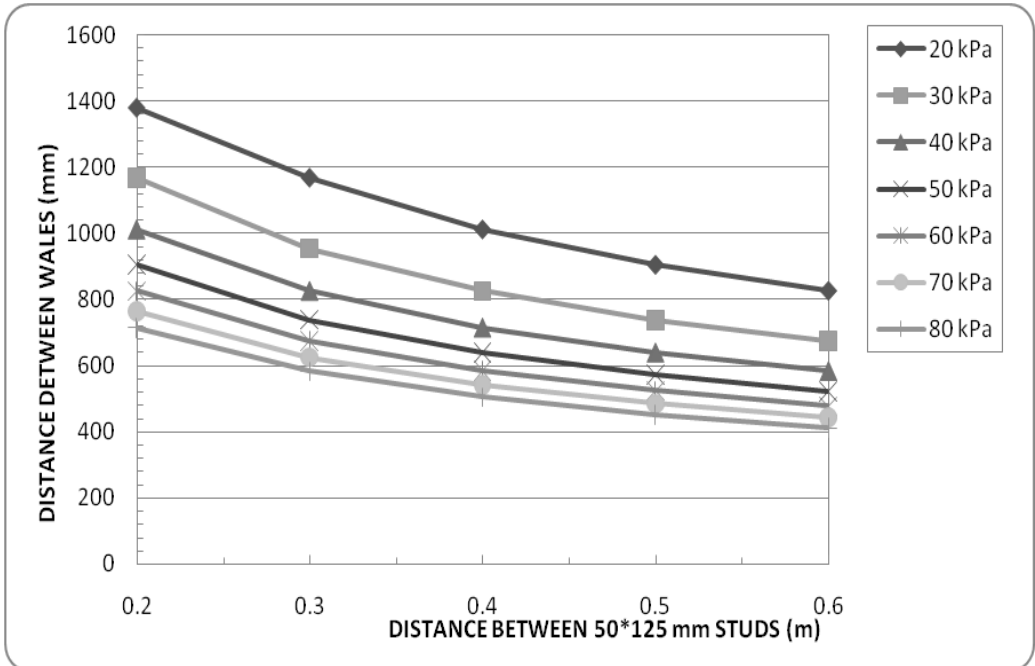


Fig. (6) Determination of safe distance between walls (50\*125 mm studs)

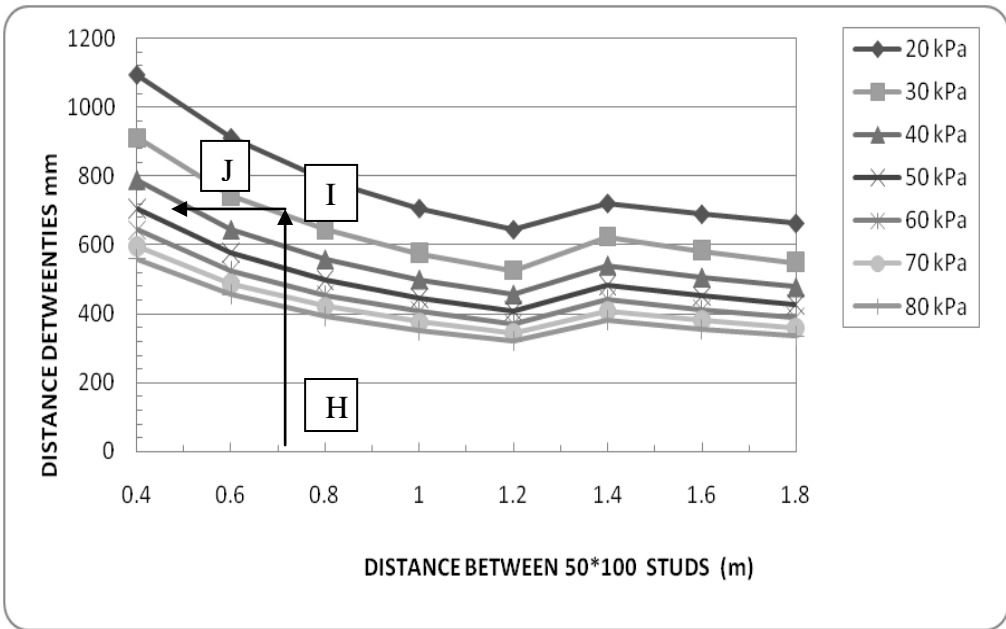


Fig. (7) Determination of safe distance between ties using double 50\*100mm (2\*4 in ) walls

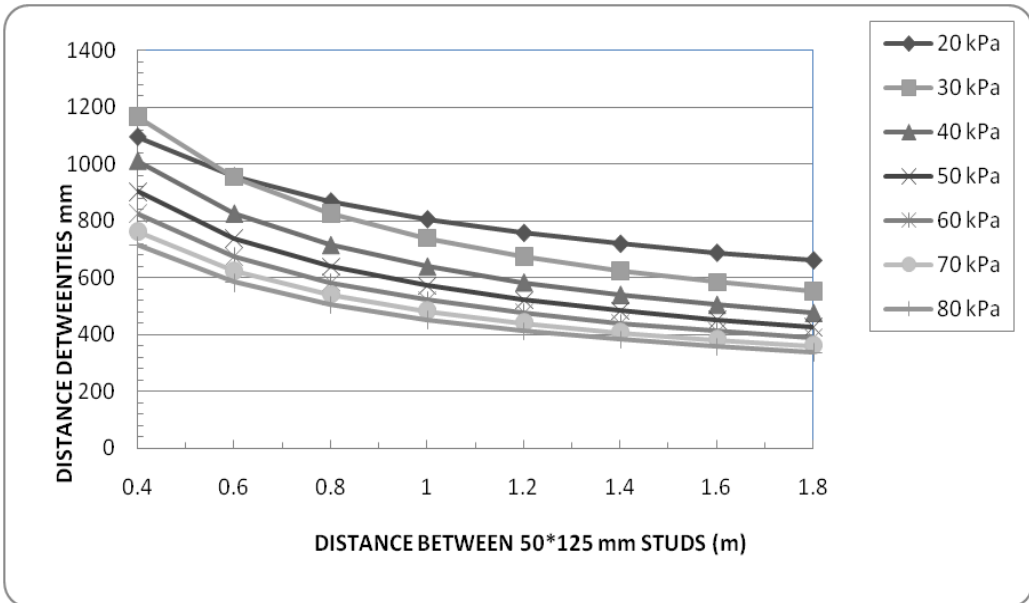


Fig.(8) Determination of safe distance between ties using double 50\*125mm (2\*5 in ) walls



## CONCLUSIONS

Design charts like the one presented here reduce most calculations to simple arithmetic. The user can easily select or check sheathing, studs, and walls without tedious calculations. It is hoped that this will help the craftsman or the small contractors to build forms with some engineering logic and not by empirical guesses.

## REFERENCES

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### منحنيات مبسطة لتصميم أنظمة الشدات الراسية

يتناول هذا البحث طريقة مبسطة لتصميم الشدات الراسية التقليدية للحوائط من الخشب باستخدام المنحنيات. في الخطوة الاولى يتم حساب الضغط الناتج عن الخرسانة الطازجة باستخدام نوع الاسمنت ووزن وحدة الحجم للخرسانة و معدل الصب ودرجة الحرارة. و في الخطوة الثانية يتم ايجاد المسافة بين العناصر الراسية باستخدام تطبيق بلاي وود سمك 19 او 22 او 25 مم. اما في الخطوة الثالثة فيتم ايجاد المسافة بين العناصر الافقية في حالة استخدام عناصر راسية خشبية 50 \* 100 او 50 \* 125 او 50 \* 150 مم. و اخيرا يتم ايجاد المسافة بين الروابط المعدنية باستخدام عناصر افقية خشبية مزدوجة عبارة عن قطاعين بابعاد 50 \* 100 او 50 \* 125 مم .